

# *“Assessment of material strain limits for defining different forms of plastic hinge region in concrete structures”*

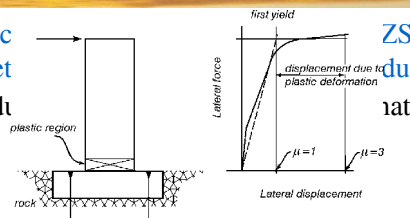
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## Outline

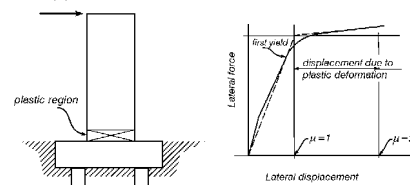
1. Background
2. Objectives
3. Experiment Details
4. Test Results and Observations
5. Calculation of Curvatures
6. Revised Limits

## Background

- Until the recent detailing design
- Structural design



(a) Structural wall with stiff foundation



(b) Structural wall with flexible foundation

ZS3101, the level of ductility  
variation in individual hinges

## Background

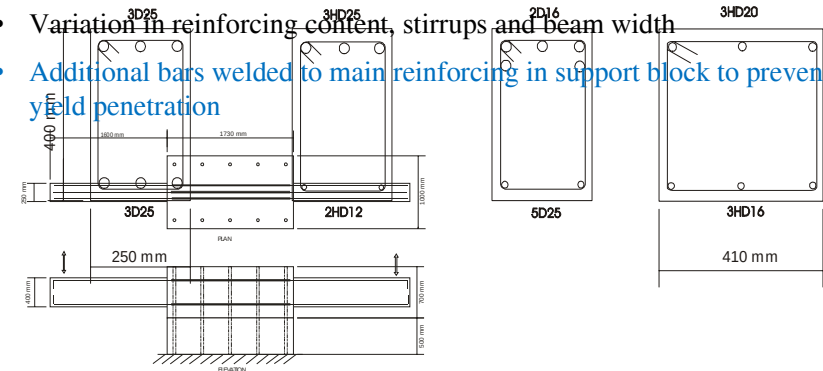
- Consequently, the revised loadings standard (NZS 1170.5:2004) requires the detailing of plastic hinges to be based on inelastic deformation demand of the hinges
- The revised concrete standard (NZS 3101:2006) classifies hinge regions based on material strains, in the form of curvatures or shear deformation.
- Plastic regions divided into three categories:
  - Nominally ductile (NDPR)
  - Limited ductile (LDPR)
  - Ductile (DPR).

## Objectives

- When 2006 code released, only limited experimental results were available
- Database improved by Fenwick and Dhakal with recommendations on limited ductile and ductile members
- Still significant gap in tests meeting nominally ductile detailing requirements
- Experimental program to fill this gap
- Revision of existing limits for ductile and limited ductile hinges

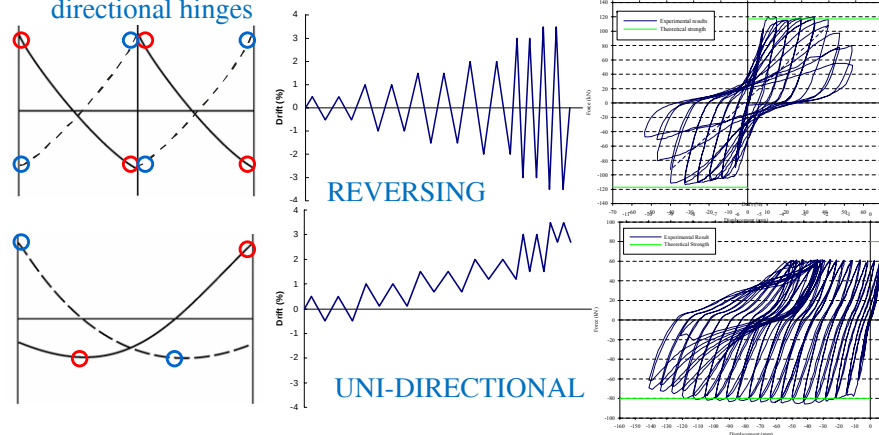
## Experimental Details

- Tests conducted on eight cantilever **NOMINALLY DUCTILE** beams
- Variation in reinforcing content, stirrups and beam width
- Additional bars welded to main reinforcing in support block to prevent yield penetration



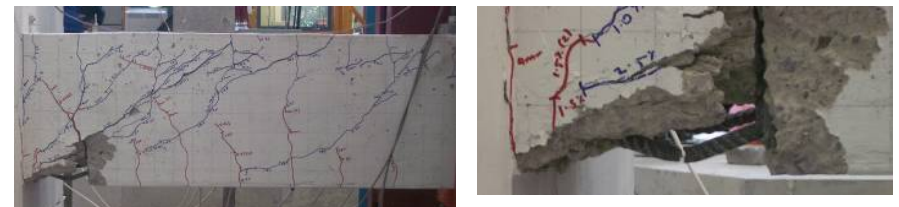
## Experimental Details

- Two different loading protocols used to form reversing and uni-directional hinges

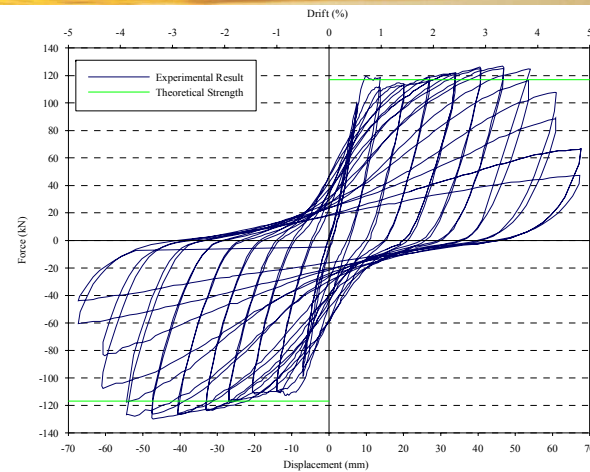


## General Observations

- Yield of reinforcing occurred on way to 1% drift except tests B1 and B2 due to crushing of concrete
- All beams formed diagonal tension cracks
- Failure due to bar buckling and leading to fracture

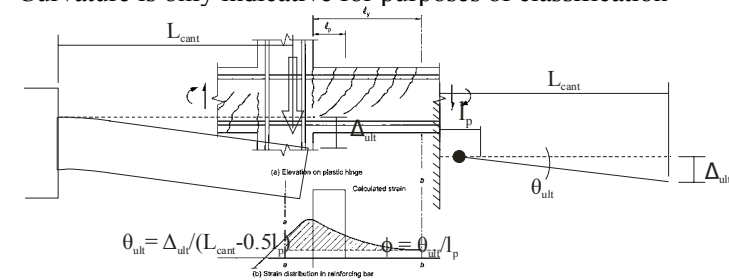


## Typical load-displacement response

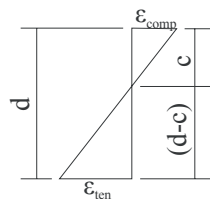


## Calculation of ultimate curvature

- Curvature is calculated as rotation from deflection profile divided by effective hinge length as defined in NZS3101
- Assumes uniform curvature over effective hinge length
- Curvature is only indicative for purposes of classification



## Calculation of curvature limit



$$\phi_{comp} = \epsilon_{comp} / c$$

$$\epsilon_{comp} = 0.004$$

$$\phi_{ten} = \epsilon_{ten} / (d - c)$$

$$\epsilon_{ten} = 0.015$$

Nominally ductile

$$\gamma = \frac{\epsilon_y}{d - c} \longrightarrow \phi_y = \frac{2\epsilon_y}{h}$$

Ductile and Limited ductile

## Design limits vs Experiment

| Test | Experiment<br>$\phi_u$ (mm <sup>-1</sup> ) | Ratio ultimate/design   |                         |
|------|--------------------------------------------|-------------------------|-------------------------|
|      |                                            | $\phi_u / \phi_{all,c}$ | $\phi_u / \phi_{all,s}$ |
| A1   | 0.000161                                   | 3.75                    | 4.70                    |
| A2   | 0.000215                                   | 5.01                    | 6.27                    |
| B1*  | 0.000108                                   | 5.24                    | 2.47                    |
| B2*+ | 0.000242                                   | 7.33                    | 3.46                    |
| C1   | 0.000134                                   | 8.47                    | 2.57                    |
| C2+  | 0.000296                                   | 11.7                    | 3.55                    |
| D1   | 0.000188                                   | 3.15                    | 5.79                    |
| D2+  | 0.000538                                   | 5.64                    | 10.4                    |

## Curvature ductility for different plastic hinges

|                      | Beams             |         | Columns           |         | Walls             |                 |                           |
|----------------------|-------------------|---------|-------------------|---------|-------------------|-----------------|---------------------------|
|                      | Nominally ductile | Ductile | Limited ductile   | Ductile | Nominally Ductile | Limited Ductile | Ductile Doubly Reinforced |
| Average              | 13.3              | 24.8    | 19.5              | 29.2    | 7.5               | 9.2             | 20.8                      |
| Std. deviation       | 5.2               | 6.9     | 4.6               | 5.6     | 3.6               | 2.2             | 3.8                       |
| Lower Characteristic | 4.7               | 13.5    | 11.9 <sub>y</sub> | 20.0    | 1.6               | 5.6             | 14.6                      |
| Number of units      | 6                 | 25      | 7                 | 9       | 5                 | 17              | 8                         |

## Design Recommendations

| Beams             |                 |         | Columns                     |         | Walls             |                  |           |
|-------------------|-----------------|---------|-----------------------------|---------|-------------------|------------------|-----------|
| Nominally Ductile | Limited Ductile | Ductile | Nominally & Limited Ductile | Ductile | Nominally ductile | Limited ductile* | Ductile** |
| 4.5               | 9               | 13.5    | 12                          | 20      | 1.5               | 5.5              | 14.5      |

## Conclusions

- Experimental tests were conducted on nominally ductile beams to provide data to set design material strain limits for this category
- Experimental results on ductile and limited ductile plastic hinges from literature were used to update the design curvature ductility for these two categories.
- An approximate method is recommended to estimate yield curvature, which does not require to determine the NA depth.
- Updated design limits have been proposed for nominally ductile plastic regions in terms of curvature ductility

## Acknowledgements

- Richard Fenwick
- EQC and CCANZ
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